

Page	1 of 8
Annexes	2
Initials	AREH/PDC

Teknologiparken Kongsvang Allé 29 DK-8000 Aarhus C Tel. +45 72 20 10 00 Fax +45 72 20 10 19 info@teknologisk.dk www.teknologisk.dk

Test report – Masonry based on lime mortar

Determining the Secant Modulus of Elasticity of Masonry based on lime mortar and measuring the long-term compressive and flexural strength of hardened lime mortar

Specimens	6 walls with the dimension 588x588x108 mm and 36 mortar prism with dimension the 160x40x40mm:
	1 Wall type A
	1 Wall type B
	1 Wall type C
	3 Walls type E
	See annex 1 for details about the brick types A, B, C and E.
	12 sets of 3 mortar prisms.
	Applied mortar: K100/1200 mortar from <i>Vejle Kalk- & Mørtelværk</i> . (Lime Mortar 7.7%).
Start date:	Wall type A, B and C were built 2013-09-05.
	6 sets of mortar prisms were built 2013-11-21.
	3 walls of type E and 6 mortar prism were cast 2014-01-08.
Method:	The walls were tested according to EN 1052-1 Methods of test for masonry – Part 1: Determination of compressive strength.
	The mortar samples were tested according to EN 1015-11 Methods of test for mortar for masonry – Part 11: Determination of flexural and compressive strength of hardened mortar.
Period:	The experiments were conducted in the period from 2013-09-05 to 2015-03-13.
Results:	See page 4-8 (table 1-4).

2015-10-30, Danish Technological Institute, Masonry, Aarhus

Arash Ehtesham Civil engineering Direct phone.: +45 7220 1481 E-mail: AREH@teknologisk.dk Poul D. Christiansen Civil engineering Direct phone.: +45 7220 3820 E-mail: PDC@teknologisk.dk



Test setup

The deformation of the wall during the compression was measured with two *HBM 50 mm transducers*. The transducers were pushed downwards onto a steel rod, which was fixated to a lightweight aluminum frame. The mean deformation in the center of the wall was determined based on the mean value of the two measured deformations on each sides of the wall. The stiffness of the frame is assumed to be infinite in the vertical direction.

In order to determine the strains, the initial distance between the two aluminum frames was measured. The test was recorded with the data aquisition hardware *HBM Spider8* and the software *CatmanEasy*.



Picture 1: Test setup.

Above and below the wall a soft 10 mm layer of masonite was placed to reduce the irregularities on the surfaces. A steel profile was mounted above the masonite layer to distribute the load equally. The weight of the distribution profile was included in the calculations. The walls were tested in an Amsler 500 ton compression machine.

Determination of E_{0k} is based on the assumed characteristic compressive strength of the masonry equal to $f_k = 3.15$ MPa - in reality, the value is higher, but this is of lesser importance. This means that E_{0k} was determined in the range of $0,05f_k$ to $0,35f_k$.

2015-04-10 AREH/PDC





Picture 2: Placement of the deformation transducer on the steel rod.

Picture 3. Overall view of the test setup.



Results

Secant Modulus of Elasticity

The results for the secant modulus of elasticity appear on table 1 and 2, while the strength values appear on table 3. The results for the hardened lime mortar prisms appear on table 4.

The values signed with an "a" were assumed to be wrong and omitted, because the deformation transducers were occasionally gushing out in sudden increase of speed, indicating that they could have been partially stuck. The deformation transducers underwent repairs and calibration before the following tests.

The tests signed with a "b" were performed until failure occurred.

The values signed with "c" are the mean of three tests repeated on the same test setup. The values for each single test can be seen in table 2 on the next page. The purpose was simply to see the difference in values between several tests which were done right after each other on the same wall and with the exact same setup. Notice that the last tests, which were performed at 429 and 554 days of curing were first tested three times in the elastic zone, before being tested to failure in the fourth test. The failure loads are shown in table 3 on the next page.

Built	Curing days	28	60	117	144	188	229	267	313	390	429	554
Date	Wall	05-02-14	04-11-13	05-05-14	27-01-14	15-07-14	22-04-14	02-10-14	15-07-14	30-09-14	13-03-15	13-03-15
05-09-13	Α		3365 ^a		1736		3120		3310	2965°		3374 ^{bc}
05-09-13	В		5533ª		2317		4665		4507°	4574 ^c		4306 ^{bc}
05-09-13	С		3968ª		2086		3184		3139°	2957°		3154 ^{bc}
08-01-14	E1			1581		1649°		1472 ^c			1443 ^{bc}	
08-01-14	E2	345 ^b										
08-01-14	E3	610 ^b										

Table 1. Secant Modulus of Elasticity in MPa for wall A, B, C, E1, E2 and E3.

Date format: dd-mm-yy

DANISH

TECHNOLOGICAL INSTITUTE

2015-04-10 AREH/PDC



Wall A	Da		
	390	554	
Test 1	3098	3420	
Test 2	2707	3361	
Test 3	3089	3342	
Mean	2965	3374	
Wall B		Days	
	313	390	554
Test 1	4527	4541	4425
Test 2	4481	4587	4271
Test 3	4512	4595	4221
Mean	4507	4574	4306
Wall C		Days	
	313	390	554
Test 1	3082	3118	3090
Test 2	3162	2835	310/
		2055	5194
Test 3	3173	2035	3174
Test 3 Mean	3173 3139	2918 2957	3174 3178 3154
Test 3 Mean Wall E1	3173 3139	2035 2918 2957 Days	3174 3178 3154
Test 3 Mean Wall E1	3173 3139 188	2033 2918 2957 Days 267	3194 3178 3154 429
Test 3 Mean Wall E1 Test 1	3173 3139 188 1681	2033 2918 2957 Days 267 1483	3194 3178 3154 429 1365
Test 3 Mean Wall E1 Test 1 Test 2	3173 3139 188 1681 1661	2918 2957 Days 267 1483 1485	3194 3178 3154 429 1365 1493
Test 3 Mean Wall E1 Test 1 Test 2 Test 3	3173 3139 188 1681 1661 1606	2033 2918 2957 Days 267 1483 1485 1448	3194 3178 3154 429 1365 1493 1472

Table 2. Modulus of Elasticity in MPA for wall A, B, C and E1.

Table 3. Strength in MPa for wall A, B, C, E1, E2 and E3.

Wall	Age at	Failure	Load area	Strength
	testing [days]	load [kN]	[mm ²]	[MPa]
Α	554	663	54864	12.1
В	554	613	54864	11.2
С	554	338	54864	6.16
E1	429	337	54864	6.14
E2	28	267	54864	4.87
E3	28	282	54864	5.14





Results for the Secant Modulus of Elasticity – graphically depicted

Graph 1. The secant modulus of elasticity over time, for wall type A, B, C & E.

Stress-strain diagrams for each test are given in Annex 2.



Mortar prisms

Initially 6 sets of prisms were cast 2013-11-21. Afterwards it was decided to add another 6 prisms to the test to gain more measurements. The last 6 sets of prisms were cast 2014-01-08. The values in table 4 are mean values¹.

Table 4: Mean compressive- and flexural strengths of lime mortar 7.7% during a period from 28 days to 421 days of curing.

Set	Cast date	Test date	Curing	Mean compressive	Mean flexural
no.	dd-mm-yyyy	dd-mm-yyyy	days	strength [MPa]	strength [MPa]
1	2014-01-08	2014-02-05	28	0.46	0.40
2	2013-11-21	2013-12-20	29	0.40	0.44
3	2013-11-21	2014-01-22	62	0.71	0.65
4	2014-01-08	2014-03-12	63	0.55	0.43
5	2014-01-08	2014-04-08	90	0.47	0.39
6	2014-01-08	2014-05-08	120	0.46	0.44
7	2013-11-21	2014-04-14	144	0.65	0.64
8	2014-01-08	2014-07-09	182	0.52	0.49
9	2014-01-08	2014-08-25	229	0.53	0.47
10	2013-11-21	2014-07-15	236	0.87	0.42
11	2013-11-21	2014-09-30	313	0.74	0.52
12	2013-11-21	2015-01-16	421	0.90	0.68
Stan	dard deviation			0.17	0.10
Coef	ficient of Variat	ion		27.6	20.7

The strength progressions are illustrated on graph 2 and 3 for the mean compressiveand flexural strength.

¹ The mean flexural strengths are based on results from three samples while the mean compressive strengths are based on results from six samples.





Graph 2: Mean compressive strengths of hardened lime mortar 7.7% during a period from 28 days to 421 days of curing.

Graph 3: Mean flexural strengths of hardened lime mortar 7.7% during a period from 28 days to 421 days of curing





SELECTED BRICKTYPES

Annex 1

The bricks are selected and purchased by the Danish Technological Institute, Masonry department.

The bricks have the Danish standard format and their initial rate of water absorption is determined as described in DS / EN 772-11 for brick type A, B, C and E. The normalized compressive strength value for brick type E is from the DoP. The pictures below illustrates the applied brick types.

Picture 1. Brick type A: "Rosé, soft-molded" from Vedstårup Teglværk A/S



Picture 2. Brick type B: "Yellow, soft-molded" from Prøvelyst Teglværk A/S



Picture 3. Brick type C: "Yellow, extruded" from Vindø Teglværk A/S





Picture 4. Brick type E: "Red, soft-molded – 2.2.07" from "Helligsø Teglværk A/S"



The values for the initial rate of water absorption and the normalized compression strength are given in the following two tables:

Specimen	Initial rate of water absorption kg/m ²				
	А	В	С	Е	
No.					
1	1.4	2.4	3.8	1.7	
2	1.3	2.5	3.0	1.7	
3	1.5	2.4	3.7	1.8	
4	1.2	2.2	3.1	1.9	
5	1.4	2.6	3.8	1.7	
6	1.3	2.3	3.2	1.7	
7	1.2	2.3	3.4	1.7	
8	1.4	2.3	3.1	1.7	
9	1.3	2.5	3.4	1.8	
10	1.1	2.4	3.7	1.7	
Mean	1.3	2.4	3.4	1.7	

Table 1. Results for the initial rate of water absorption for brick type A, B, C and E.

Table 2.	Results for	• the normalized	compression	strength for	brick type A, B,	C and E
10000 -0	100000000000000000000000000000000000000	nie nennaden jeur	compression	serengen jer	<i>orrent type</i> 11, 2,	e unter B

Normalized compression strength MPa					
27	32	42	16^{*}		
	Norn A 27	Normalized com M A B 27 32	Normalized compression streMPaABC273242		

*Value from DoP – has not been tested by DTI.













































13















